GENERAL NOTES:

****
ALL NOTES HEREAFTER ARE TYPICALLY APPLICABLE UNLESS OTHERWISE NOTED ON PLANS OR DETAILS.
****

GENERAL:

G 1 THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECT'S PLANS BEFORE STARTING WORK.

G 2 SEE ARCHITECTURAL PLANS FOR EXACT DIMENSIONS AND LOCATIONS OF OPENINGS IN WALLS, ROOF AND FLOOR SYSTEMS.

G 3 THE CONTRACTOR SHALL VERIFY ALL MECHANICAL OPENING SIZES, LOCATIONS AND UNIT WEIGHTS WITH MECHANICAL CONTRACTOR.

G 4 SUBMIT ALL SHOP DRAWINGS ON ONE REPRODUCIBLE PRINT AND TWO BLUE-LINE PRINTS. THE REPRODUCIBLE PRINT WILL BE RETURNED. ALL BLUE-LINE PRINTS REQUIRED BY THE CONTRACTOR ARE THE RESPONSIBILITY OF THE CONTRACTOR.

G 5 GENERAL CONTRACTOR SHALL APPROVE AND STAMP ALL SHOP DRAWINGS BEFORE SUBMITTING FOR STRUCTURAL REVIEW. ALL DEVIATIONS FROM THE CONTRACT DRAWINGS SHALL BE HIGHLIGHTED BY THE FABRICATOR AND GENERAL CONTRACTOR.

G 6 CONTRACTOR SHALL PROVIDE AND MAINTAIN TEMPORARY BRACING, SHORING, GUYING, ETC. AND OTHER METHODS TO PREVENT EXCESSIVE STRESSES AND HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THESE PROVISIONS TO REMAIN IN PLACE UNTIL SUFFICIENT PERMANENT MEMBERS ARE CONSTRUCTED TO INSURE THE SAFETY OF THE STRUCTURE.

G 7 ALL DETAILS SHOWN ARE TYPICAL. SIMILAR DETAILS APPLY TO SIMILAR CONDITIONS, UNLESS SHOWN OTHERWISE

FOUNDATION/SOILS:

FS 1 SEE SOILS REPORT FOR PREPARATION OF SOILS AT BUILDING PAD. ALL SOILS WORK, INCLUDING BACKFILL OF UTILITY TRENCHES AND VERIFICATION OF BEARING CAPACITY, SHALL BE UNDER THE DIRECTION OF A QUALIFIED SOILS ENGINEER. PROXIMITY OF UTILITY TRENCHES TO BUILDING FOUNDATION SYSTEM SHALL BE AS APPROVED BY THE ARCHITECT AND/OR SOILS ENGINEER TO INSURE THE INTEGRITY OF THE BEARING SOILS.

FS 2 ALL FOUNDATIONS SHALL BEAR ON UNDISTURBED EARTH OR ENGINEERED FILL AT ELEVATIONS SHOWN ON DETAILS. BEARING MATERIALS TO BE VERIFIED BY A LICENSED SOILS ENGINEER.

FS 3 ALL ABANDONED FOUNDATIONS, UTILITIES, ETC., THAT INTERFERE WITH NEW CONSTRUCTION SHALL BE REMOVED. BEAR NEW FOUNDATIONS ON ENGINEERED FILL OR INCREASE BEARING DEPTH TO UNDISTURBED EARTH AT REMOVED OBSTRUCTIONS.

FS 4 ALL FOUNDATIONS (OR PORTIONS THEREOF) BELOW GRADE MAY BE EARTH FORMED BY NEAT EXCAVATIONS, SOIL PERMITTING.

FS 5 FOOTINGS TO BE CENTERED ON WALLS, OR COLUMNS, U.N.O.

FS 6 GRADE BEAM CONSTRUCTION JOINTS TO BE CENTERED BETWEEN PIERS.

FS 7 FOUNDATION DESIGN IS BASED ON GEOTECHNICAL ENGINEERING REPORT PREPARED BY TERRACON, PROJECT No. ??????, DATED ????????.

CONCRETE:

C 1 CONCRETE REINFORCING FABRICATOR AND GENERAL CONTRACTOR SHALL CERTIFY THAT THEY HAVE REVIEWED ALL SHOP DRAWING SHEETS BEFORE SUBMITTING FOR REVIEW, BY STAMPING EACH SHEET, OR BY COMPANY LETTERHEAD STATING THE SAME, FOR EACH SUBMITTAL. ALL DEVIATIONS FROM THE CONTRACT DRAWINGS SHALL BE HIGHLIGHTED BY THE FABRICATOR AND/OR GENERAL CONTRACTOR. SHOP DRAWINGS SUBMITTED FOR ENGINEER'S REVIEW ARE ONLY CHECKED FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. NO RESPONSIBILITY IS ASSUMED BY THE ENGINEER FOR THE CORRECTNESS OF DIMENSIONS, DETAILS, QUANTITIES OR PROCEDURES SHOWN ON THE SHOP DRAWINGS.

C 2 SHOP DRAWINGS SHALL BE PREPARED IN KEEPING WITH THE CURRENT NATIONAL STANDARDS: A.C.I. AND C.R.S.I.

C 3 CONCRETE SHALL CONFORM TO THE CURRENT "ACI MANUAL OF CONCRETE PRACTICE".

C 4 ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. SEE PLANS AND DETAILS FOR ADDITIONAL STRENGTHS AND REQUIREMENTS.

C 5 PORTLAND CEMENT SHALL MEET ASTM C150, TYPE I OR III.

C 6 ALL AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C33.

C 7 NO ADMIXTURES, OTHER THAN AIR ENTRAINMENT, SHALL BE USED WITHOUT THE ARCHITECT'S OR ENGINEER'S PRIOR APPROVAL.

C 8 REINFORCING SHALL BE DETAILED, FABRICATED, AND PLACED PER CRSI AND ACI. INCLUDING CONCRETE COVER AND BAR SUPPORTS. (DESIRED METHOD OF SUPPORTING TOP BARS IN THICK MATS TO BE VERIFIED WITH CONTRACTOR.) PROVIDE CORNER BARS AT ALL "TRENCH FOOTING" AND WALL INTERSECTIONS INCLUDING THICKENED SLABS. CORNER BARS TO MATCH HORIZONTAL REINFORCING IN QUANTITY, SIZE AND SPACING. AT INTERSECTIONS OF CONTINUOUS OR SPREAD FOOTINGS, EXTEND ALL BARS TO FAR SIDE OF INTERSECTING FOOTING.

C 9 LAP BARS AT ALL SPLICES AND DOWELS. MINIMUM LAP AS SCHEDULED BELOW, UNLESS NOTED OTHERWISE ON PLANS AND/OR DETAILS. STAGGER ALL SPLICES 24" MINIMUM:

BAR SIZE	VERTICAL BARS	HORIZONTAL BARS	CORNER BARS
#6 AND SMALLER	36 BAR DIA,	46 BAR DIA,	33 BAR DIA.
#7 AND LARGER	45 BAR DIA.	58 BAR DIA.	42 BAR DIA.

C 10 LAP WELDED WIRE FABRIC 6" OR ONE FULL MESH, WHICHEVER IS GREATER.

C 11 ALL REINFORCING SHALL MEET ASTM A615, GRADE 60. REINFORCING TO BE WELDED SHALL MEET ASTM A706, GRADE 60. ALL WELDED WIRE FABRIC (WWF) SHALL MEET ASTM A185.

C 12 CONCRETE PROTECTION FOR REINFORCING: 3" AT FOUNDATIONS; 2" AT FORMED SURFACES LATER EXPOSED TO SOIL OR WEATHER; 1 1/2" AT ALL OTHER FORMED SURFACES; 3/4" AT SLABS.

C 13 WELDING OF REINFORCEMENT SHALL BE WITH LOW HYDROGEN ELECTRODES IN CONFORMANCE WITH "WELDING REINFORCING STEEL, ETC.", AWS D12.1. REINFORCING TO BE WELDED SHALL MEET ASTM A706, GRADE 60.

C 14 ALL CONCRETE SLABS SHALL HAVE FROM 4% TO 6% ENTRAINED AIR.

C 15 ALL SLABS ON GRADE SHALL BE 4" THICK WITH 6x6 W2.1/W2.1 WWF AT MID-DEPTH. SEE ARCHITECTURAL PLANS FOR FINISH ELEVATIONS. SEE DETAILS FOR CONTROL AND/OR CONSTRUCTION JOINTS.

C 16 FLOOR SLABS ON GRADE SHALL HAVE SAWN CONTROL JOINTS LOCATED TO FORM APPROXIMATELY SQUARE PANELS OF NOT MORE THAN 169 SQUARE FEET. CONSTRUCTION JOINTS MAY BE SUBSTITUTED AT CONTRACTORS OPTION. CONTROL JOINTS SHALL BE CENTERED ON COLUMN CENTERLINES WHEREVER POSSIBLE.

C 17 SLABS OVER COMPOSITE BEAMS TO BE NORMAL WEIGHT CONCRETE, 8" THICK INCLUDING DECK, WITH #5 AT 12"O.C. PLACED 3 1/2" FROM TOP OF SLAB.

C 18 NO HOLES OR OPENINGS THROUGH FOUNDATION WALLS AND/OR FOOTINGS ARE PERMITTED WITHOUT ENGINEERS APPROVAL.

C 19 NO ALUMINUM SHALL BE EMBEDDED IN ANY CONCRETE.

C 20 PROVIDE 2-#5, 4'-0" LONGER THAN OPENING DIMENSION, ON ALL SIDES OF OPENINGS IN ALL CONCRETE.

C 21 LOCATION OF CONSTRUCTION JOINTS, RUSTICATION JOINTS AND BRICK LEDGES SHALL BE APPROVED BY ARCHITECT OR ENGINEER.

C 22 EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4".

STEEL:

S 1 STEEL FABRICATOR AND GENERAL CONTRACTOR SHALL CERTIFY THAT THEY HAVE REVIEWED ALL SHOP DRAWING SHEETS BEFORE SUBMITTING FOR REVIEW, BY STAMPING EACH SHEET, OR BY COMPANY LETTERHEAD STATING THE SAME, FOR EACH SUBMITTAL. ALL DEVIATIONS FROM THE CONTRACT DRAWINGS SHALL BE HIGHLIGHTED BY THE FABRICATOR AND/OR GENERAL CONTRACTOR. SHOP DRAWINGS SUBMITTED FOR ENGINEER'S REVIEW ARE ONLY CHECKED FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. NO RESPONSIBILITY IS ASSUMED BY THE ENGINEER FOR THE CORRECTNESS OF DIMENSIONS, DETAILS, QUANTITIES OR PROCEDURES SHOWN ON THE SHOP DRAWINGS.

S 2 SHOP DRAWINGS SHALL BE PREPARED IN KEEPING WITH THE CURRENT NATIONAL STANDARDS: A.I.S.C.

S 3 ELEVATIONS TOP OF STEEL NOTED ON DRAWINGS. BEAMS FRAME FLUSH AT TOP UNLESS NOTED THUS (+/-).

S 4 NO SUBSTITUTION OF MEMBERS WITHOUT ENGINEER'S APPROVAL.

S 5 STRUCTURAL STEEL SHALL MEET THE LATEST AISC "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".

S 6 ALL WIDE FLANGE AND WT STEEL SHAPES SHALL MEET ASTM A992(Fy = 50 KSI).
ALL TUBING SHALL MEET ASTM A500, GR. B (Fy=46 KSI).
ALL PLATES, ANGLES, AND MISC. STEEL SHALL MEET ASTM 36 (Fy=36 KSI).

S 7 BOLTS SHALL BE A325N, WITH WASHERS AS REQUIRED (EXCEPT ANCHOR BOLTS SHALL BE A307).

S 8 ALL FIELD CONNECTIONS SHALL BE BOLTED UNLESS NOTED OTHERWISE.

S 9 WELDING SHALL CONFORM TO THE STANDARDS SET FORTH IN THE AWS PUBLICATION "WELDING IN BUILDING CONSTRUCTION". WELDS SHALL BE MADE USING E70xx ELECTRODES. MINIMUM FIELD AND SHOP WELD SIZE TO BE 1/4" FILLET WELD, UNLESS NOTED OTHERWISE.

S 10 ERECTION DRAWINGS SHALL SHOW ALL FIELD WELDS REQUIRED.

S 11 NO OPENINGS TO BE PLACED IN BEAM WEBS OR FLANGES WITHOUT ENGINEER'S APPROVAL.

S 12 ALL BEAMS SHALL HAVE STANDARD AISC FRAMING CONNECTIONS.

S 13 BEAMS WHICH ARE COMPOSITE SHALL HAVE 1/2" DIAMETER x 4" STUDS; NUMBER REQUIRED NOTED THUS (xx) ON PLANS. STUDS SHALL BE INSTALLED IN STRICT COMPLIANCE WITH MANUFACTURER'S RECOMMENDATIONS. SPACE STUDS UNIFORMLY ALONG BEAM WITHIN AREAS INDICATED ON PLANS. PLACE STUDS DIRECTLY OVER BEAM WEB AT DECK FLUTES WITH ONE STUD AND SYMMETRICALLY AT DECK FLUTES WITH MORE THAN ONE STUD.

S 14 ALL STEEL DECK SHALL BE WELDED TO ALL MEMBERS ON WHICH IT BEARS.

S 15 ALL STEEL ROOF DECK SHALL BE, 1 1/2", 22 GA., WIDE RIB, PAINTED. ERECT PER MANUFACTURER'S SPECIFICATION.

S 16 MIN. ROOF DECK ATTACHMENT TO BE 5/8" DIAMETER PUDDLE WELDS AT END AND PARALLEL SUPPORTS. SIDELAP FASTENERS TO BE #10 TEK SCREWS.

S 17 SHOP DRAWINGS SHALL SHOW ALL DECK FASTENERS, DIAPHRAGM SHEAR CAPACITIES, UPLIFT CAPACITIES AND ATTACHMENT PATTERNS.

S 18 OPENING FRAMES FOR ROOF SHALL BE FABRICATED FROM L4x3x1/4 (LONG LEG DOWN) ANGLES; EXTEND TO JOISTS, BLOCK VERTICAL LEG, FIELD WELD.

S 19 COMPOSITE SLABS SHALL BE ON 1.5"x20 GAUGE GALVANIZED COMPOSITE DECK. (W/H>=2). SHORING AND WELD REQUIREMENTS SHALL BE SHOWN ON ERECTION DRAWINGS. INSTALLATION TO BE IN STRICT COMPLIANCE WITH MANUFACTURER'S RECOMMENDATIONS. DECK SHALL BE WELDED TO ALL MEMBERS IN WHICH IT BEARS AT 16" O.C. MAX. DESIGN LOAD AND SHORING TABLES SHALL BE SUBMITTED WITH SHOP DRAWINGS.

MASONRY:

M 1 MASONRY REINFORCING FABRICATOR AND GENERAL CONTRACTOR SHALL CERTIFY THAT THEY HAVE REVIEWED ALL SHOP DRAWING SHEETS BEFORE SUBMITTING FOR REVIEW, BY STAMPING EACH SHEET, OR BY COMPANY LETTERHEAD STATING THE SAME, FOR EACH SUBMITTAL. ALL DEVIATIONS FROM THE CONTRACT DRAWINGS SHALL BE HIGHLIGHTED BY THE FABRICATOR AND/OR GENERAL CONTRACTOR. SHOP DRAWINGS SUBMITTED FOR ENGINEER'S REVIEW ARE ONLY CHECKED FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT OF THE PROJECT AND GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. NO RESPONSIBILITY IS ASSUMED BY THE ENGINEER FOR THE CORRECTNESS OF DIMENSIONS, DETAILS, QUANTITIES OR PROCEDURES SHOWN ON THE SHOP DRAWINGS.

M 2 SHOP DRAWINGS SHALL BE PREPARED IN KEEPING WITH THE CURRENT NATIONAL STANDARDS: N.C.M.A.

M 3 HOLLOW CONCRETE BLOCK (MASONRY) UNITS SHALL MEET ASTM C90, NORMAL WEIGHT, TYPE 1 WITH A MINIMUM COMPRESSIVE STRENGTH OF 2670 PSI ON THE NET AREA ($f'm$ = 2000 PSI).

M 4 NO "U" TYPE LINTEL BLOCKS SHALL BE USED FOR BOND BEAMS, U.N.O. USE OPEN BOTTOM STANDARD BOND BEAM BLOCK WITH NOTCHED KNOCK-OUT WEBS AND EXPANDED METAL LATH GROUT STOPS.

M 5 MASONRY DESIGN STRENGTH ($f'm$) SHALL BE DETERMINED BY PRISM TESTS CONFORMING TO ASTM E447, METHOD B, IN ADVANCE OF BEGINNING MASONRY WORK. STANDARD AGE OF TEST SPECIMENS SHALL BE 28 DAYS.

M 6 MASONRY WORK SHALL HAVE SPECIAL INSPECTION AS DEFINED BY THE GOVERNING BUILDING CODE.

M 7 NO STACKED BOND MASONRY CONSTRUCTION SHALL BE ALLOWED.

M 8 MORTAR FOR USE IN MASONRY SHALL MEET ASTM C270, TYPE M OR S.

M 9 GROUT FOR USE IN MASONRY SHALL MEET ASTM C476; COMPRESSIVE STRENGTH 2500 PSI FOR $f'm$ = 2000 PSIAT 28 DAYS WITH NOT LESS THAN 6 1/2 SACKS OF CEMENT PER YARD, COURSE SAND OR PEA GRAVEL (3/8" MAX. AGGREGATE), AND A SLUMP OF NOT GREATER THAN 8".

M 10 VERTICAL GROUT LIFT OPTIONS:
LOW-LIFT GROUTING:
GROUT LIFTS SHALL NOT EXCEED 4'-0" MAX. THE TOP OF EACH GROUT LIFT SHALL BE 2" BELOW THE TOP OF THE MASONRY UNIT. ALL AIR POCKETS SHALL BE REMOVED BY RODDING OR VIBRATING THE GROUT.
HIGH-LIFT GROUTING:
PROVIDE CLEANOUTS AT EACH BAR LOCATION. GROUT POURS SHALL NOT EXCEED 6'-0".
SEE MINIMUM CMU WALL REINFORCEMENT DETAIL A/S2 FOR ADDITIONAL INFORMATION.

M 11 MASONRY BEARING WALLS SHALL BE LAID WITH ALL FACE SHELLS SOLIDLY IN MORTAR.

M 12 REINFORCING BARS SHALL MEET ASTM A615 GR. 60 (FY=60,000 PSI). ALL CELLS CONTAINING REINFORCEMENT SHALL BE SOLID GROUTED.

M 13 PROVIDE A MINIMUM OF 1/2" GROUT BETWEEN MAIN REINFORCING AND MASONRY UNITS.

M 14 HOLD VERTICAL BARS STRAIGHT AND TRUE AND CENTERED IN WALL.

M 15 PROVIDE DOWELS FROM FOUNDATIONS AT ALL VERTICAL MASONRY WALL REINFORCEMENT. DOWELS SHALL MATCH VERTICAL REINFORCEMENT IN SIZE AND SPACING, UNLESS NOTED OTHERWISE ON PLANS OR DETAILS. PROVIDE ONE DOWEL FOR EACH VERTICAL WALL BAR AT EACH LOCATION.

M 16 HORIZONTAL AND VERTICAL REINFORCEMENT SHALL BE CONTINUOUS AND LAPPED A MINIMUM NUMBER OF BAR DIAMETERS AS SCHEDULED:

	SINGLE REINFORCING	DOUBLE REINFORCING AND BOND BEAMS
#4'S	43 BAR DIA.	51 BAR DIA.
#5'S	43 BAR DIA.	65 BAR DIA.
#6'S	66 BAR DIA	113 BAR DIA.

M 17 EXCEPT AS NOTED ON PLANS OR DETAILS, REINFORCE MASONRY WALLS TYPICALLY AS FOLLOWS:
VERTICAL:
ALL 8" MASONRY WALLS UNO:
(1)#5 AT 24" O.C. PLUS REINFORCING SPECIFIED ABOVE FOR "ALL MASONRY WALLS".
ALL 8" MASONRY WALLS NOTED THUS 'MW1':
(1)#5 AT 8" O.C. PLUS REINFORCING SPECIFIED ABOVE FOR "ALL MASONRY WALLS".
ALL 12" MASONRY:
(2)#5 AT 8" O.C. PLUS REINFORCING SPECIFIED ABOVE FOR "ALL MASONRY WALLS".
HORIZONTAL: #9 GAGE WIRE LADDER TYPE JOINT REINFORCEMENT AT 16" O.C. AND 2-#5 CONTINUOUS IN 8" DEEP BOND BEAM AT FLOOR LINES, AT 7'-4" ABOVE FINISHED FLOOR, ROOF LINES AND AT THE TOP OF PARAPET.

M 18 PROVIDE 1-#5 VERTICAL MIN. AT ALL WALL ENDS, CORNERS, AND CONTROL JOINTS AND AT THE INTERSECTION OF ALL WALLS, SEE DETAIL.

M 19 PROVIDE 2-#5 MIN. IN 8" DEEP BOND BEAM AT HEAD AND SILL OF ALL OPENINGS IN MASONRY WALLS. EXTEND 48 BAR DIAMETERS PAST EDGE OF OPENING.

M 20 REINFORCE MASONRY AT BEARING POINTS OF ALL BEAMS, LINTELS, JOIST GIRDERS, AND TRUSSES WITH 1-#5 VERTICAL MIN. (CONTINUOUS TO FOUNDATION) IN EACH CELL BENEATH BEARING PLATES.

M 21 SEE ARCHITECTURAL FOR LOCATIONS OF CONTROL JOINTS. CONTROL JOINT SPACING SHALL NOT EXCEED TWO TIMES THE WALL HEIGHT. PROVIDE CONTROL JOINTS AT ALL INTERSECTIONS OF LOAD BEARING AND NON-LOAD BEARING WALLS, TYPICAL.

SPECIAL INSPECTIONS

ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE BUILDING CODE AND ALL LOCAL ORDINANCES. THE OWNER SHALL HIRE AN EXPERIENCED, QUALIFIED INSPECTOR TO PERFORM ALL THE REQUIRED INSPECTION WORK. THE ENGINEER WILL NOT PERFORM THE REQUIRED INSPECTION AS PART OF HIS DESIGN SERVICE. THE ENGINEER MAY VISIT THE SITE TO ASCERTAIN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS AND SUCH VISITS ARE NOT TO BE CONSTRUDED AS MEETING INSPECTION REQUIREMENTS UNLESS THE ENGINEER SPECIFICALLY SO STATES IN WRITING.

GOVERNING CODE: 2015 IBC

DESIGN LIVE LOADS
ROOF: 20 PSF

SNOW LOAD:
ROOF SNOW LOAD, Pf: 15 PSF
EXPOSURE COEFFICIENT, Ce: 0.9
IMPORTANCE FACTOR, I: 1.0
THERMAL FACTOR, Ct: 1.0

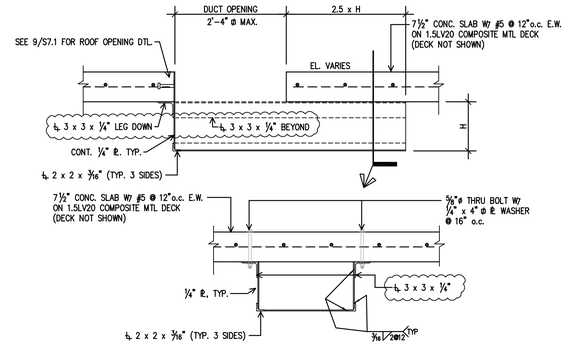
WIND LOADS
Vult: 120 MPH
IMPORTANCE FACTOR, I: 1.0
EXPOSURE: C

DESIGN UPLIFT TO ROOF DECK AND JOISTS:
FIELD: 15 PSF / 72 PSF @ SHELTER
EDGE: 29 PSF / 94 PSF @ SHELTER
CORNER: 47 PSF / 117 PSF @ SHELTER
EDGE/CORNER DISTANCE: EDGE: 10 FEET

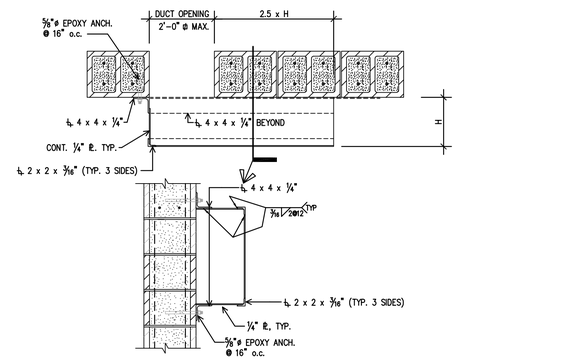
WIND LOADS (STORM SHELTER) DESIGN PER FEMA 361, 2ND EDITION
V ULT: 250 MPH
IMPORTANCE FACTOR, I: 1.15
EXPOSURE: C
DIRECTIONAL FACTOR, Kd: 1.0
INTERNAL PRESSURE COEFFICIENT, GCpi: +/-0.55

SEISMIC:
USE GROUP: I
DESIGN CATEGORY B
IMPORTANCE FACTOR, I: 1.0
S_S: 0.131
S_1: 0.067
S_{DS}: 0.139
S_{D1}: 0.107
SITE CLASS: D
RESPONSE COEFFICIENT, Cs 0.055
RESPONSE MODIFICATION, R 2.5
BASIC SEISMIC-FORCE-RESISTING SYSTEM: ORD. MAS. SHEARWALL
ANALYSIS PROCEDURE: E.L.F.

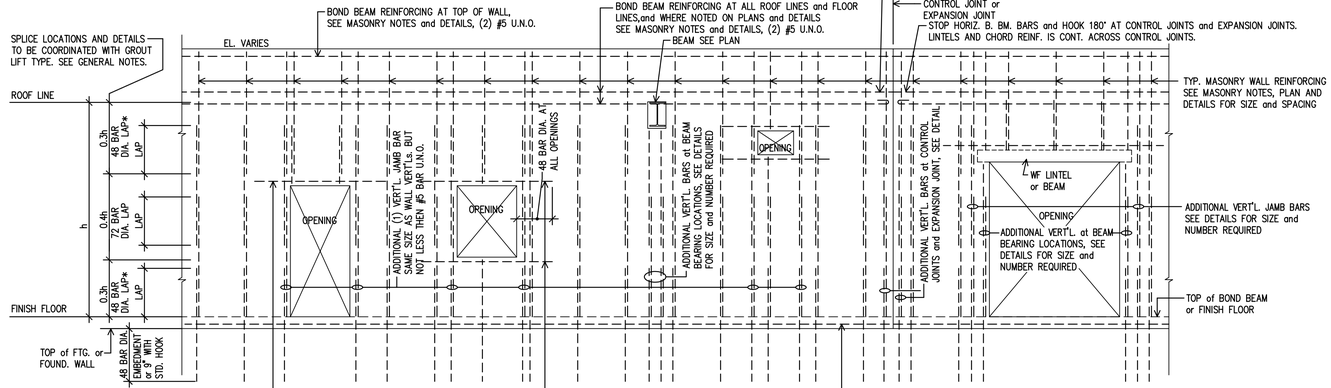
NET DESIGN SOIL PRESSURE: 2500PSF



1 SHELTER ROOF PENETRATION
3/4" = 1'-0"

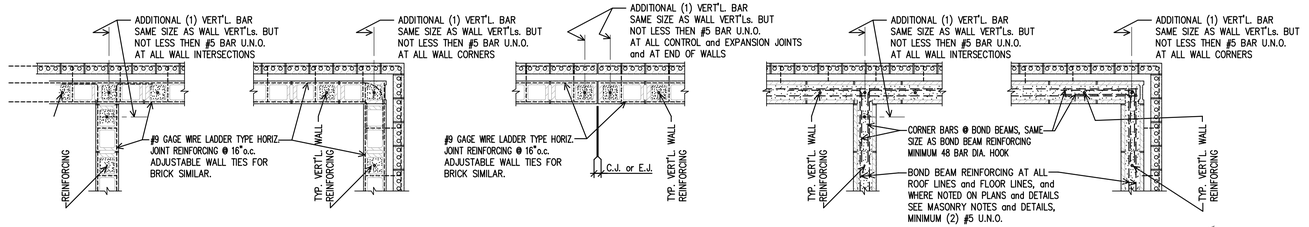


2 SHELTER WALL PENETRATION
3/4" = 1'-0"



NOTE:
ALL BOND BEAM REINFORCING SPLICES SHALL BE STAGGERED, TYP.
* AT 8" CMU with (2) VERT'LS PER CELL, LAP 62 BAR DIAMETERS OR STAGGER SPLICES AND LAP 48 BAR DIAMETERS

A MINIMUM CMU WALL REINFORCING DETAIL
NO SCALE



NOTE: MASONRY ANCHOR BOLT OPTIONS
1) CAST - IN PLACE 3/4"ø A307 BOLTS
2) EPOXY 3/4"ø A307 with 6 5/8" EMBEDMENT, USE HY150 EPOXY
5" @ 8" CMU
8¾" @ 12" CMU

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2020 UDALL HIGH SCHOOL
FEMA SHELTER ADDITION
UNIFIED SCHOOL DISTRICT #463

DRAWINGS ISSUED		
NO.	DATE	ITEM ISSUED
8	3/3	ADDENDUM
7	2/2	ISSUED FOR BIDS
6	1/11	BOE REVIEW
5	11/24	BOE REVIEW
4	11/9	BOE REVIEW
3	10/26	REVIEW
2	10/16	REVIEW
1	10/08	REVIEW

COMPUTER DRAWING

DATE: JANUARY 2021	
DRAWN BY:	CHECKED BY:
PS	PS

SHEET
S0.1
OF 1 SHEETS